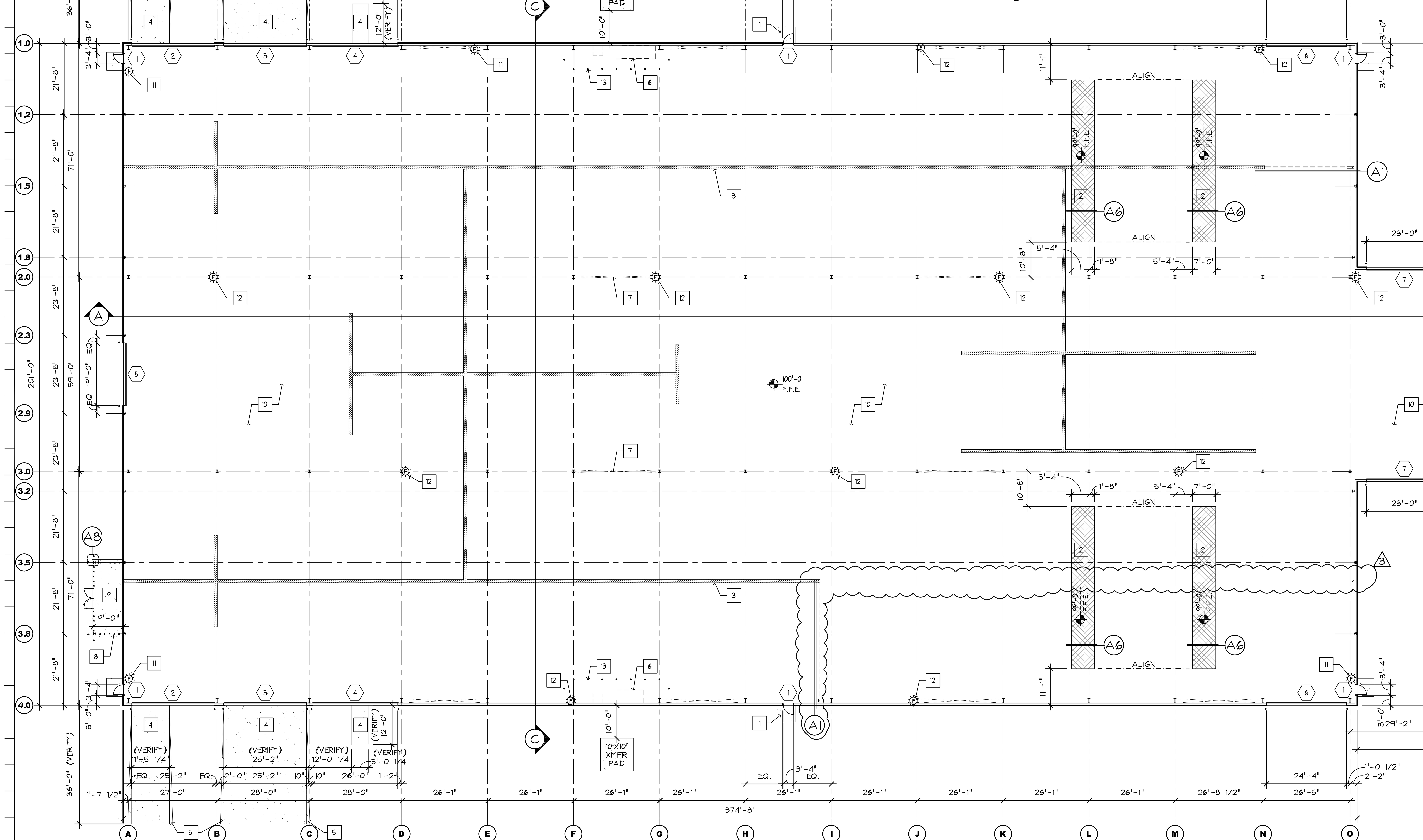

LEGEND

- DOOR PER SCHEDULE - SEE SHEET 1261
- DETAIL TAG - SEE SHEET 1251
- BUILDING SECTION TAG - SEE SHEET 1241

KEY NOTES

1. DOOR STOOP, TYP. OF 7 - SEE DTL. A7
2. PIT - SEE DTL. A6
3. UTILITY TRENCH, TYP. AS SHOWN - SEE DTL. A2
4. MATERIAL STAGING SLAB W/ TURNDOWN. MAINTAIN F.F.E. THRU-OUT SLAB. SLAB PROFILE TO MATCH BLDG. SLAB (BY OWNER)
5. SLOPE EA. SIDE OF THESE MATERIAL STAGING SLABS @ 1:4 AS SHOWN TO MEET SITE PAVING (BY OTHERS)
6. ELEC. EQUIP. - SEE ELEC. DRWGS.
7. P.E.B. CROSS BRACING LOCATION, TYP. AS SHOWN
8. CHAINLINK FENCE/GATE ENCLOSURE - SLAB TO STRUCTURE W/ DBL. 3'-0"W.x7'-0"H. GATE AS SHOWN
9. 10'-0"W.x24'-0"L. 4" CONC. SLAB ON 4" ABC W/ TURNDOWN EDGES
10. TYP. CLASS 5 FINISH CONC. SLAB ON GRADE PER ACI 302.1 R5 - SEE STRUCT. DRWGS. & SPEC SECTION 03300
    - OWNER OPTIONAL FLOOR FINISH:
      - FLOOR HARDENER APPLICATION & DUST PROOFING
      - VAPOR RETARDANT UNDER FLOOR SLABS
      - SEALANT APPLIED @ SAWED JOINTS
11. BRACKET MOUNTED FIRE EXTINGUISHER MIN. 2A:10BC - SEE DTL. A3
12. BRACKET MOUNTED FIRE EXTINGUISHER MIN. 2A:10BC - SEE DTL. A4
13. STL. BOLLARDS @ 48" O.C. TO PROVIDE 36" CLR. OF ELEC. EQUIP. PER DTL. A9

FLOOR PLAN

SCALE: 1/16" = 1'-0"

NORTH

NO.	DATE	DESCRIPTION	BY	BY	BY
1	15.2.12	1ST REVIEW & SHEET NUMBERS			PK 15/2
3	26.3.12	OWNER REVISIONS			PK 26/3

REVISIONS

PE STAMP

LICENSED ARCHITECT PATRICK J. KLENK No. C-27757 Exp 4/30/13 STATE OF CALIFORNIA

03/26/2012

WESTAR ARCHITECTS

WESTAR Architectural Group/Nevada, Inc. ARCHITECTURE • PLANNING • INTERIOR DESIGN

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ABENER TEYMA
MOJAVE

ISSUE DATE	PURPOSE OF ISSUE
14/03/12	FOR CONSTRUCTION

ABENER TEYMA MOJAVE

ABENGOA SOLAR

FLOOR PLAN

MOJAVE SOLAR, LLC HINKLEY, CALIFORNIA

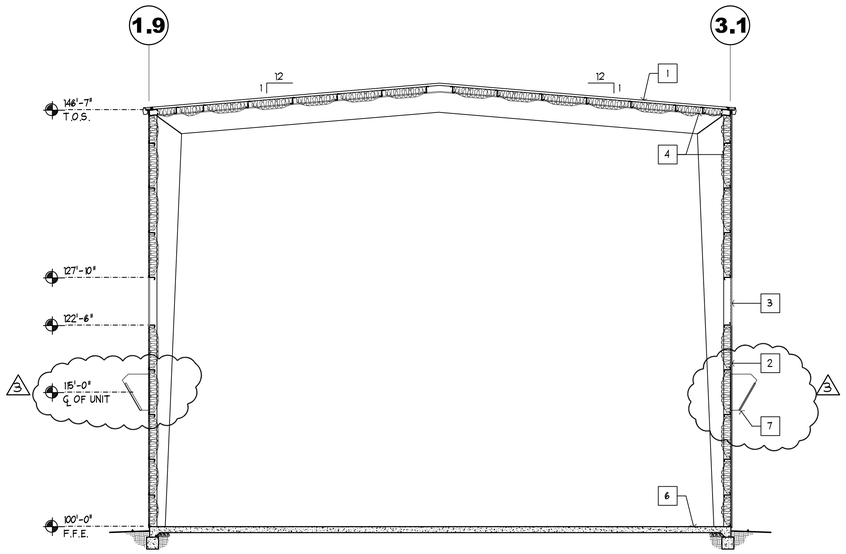
	BY	DATE	APP'D.	DATE	JOB NO.	REVISION
DRAWN	JC	18/01/12	JC	18/01/12	120010	3
CHECKED	RB	18/01/12	RB	18/01/12		
SCALE	AS NOTED					

DRAWING NUMBER

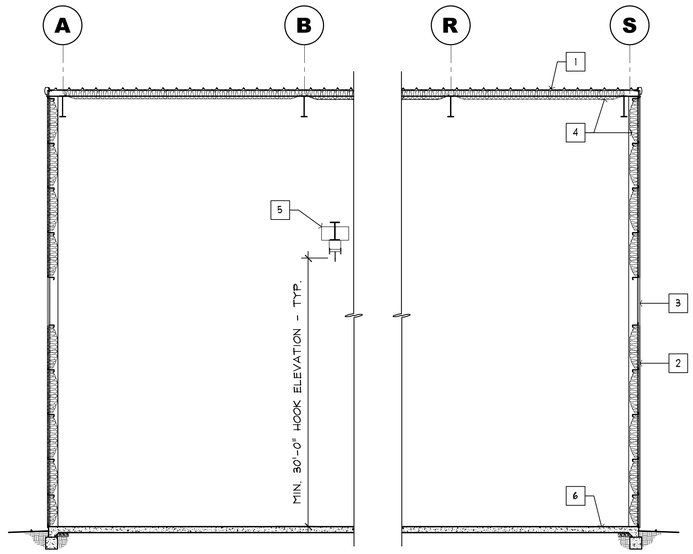
6007-PLN-CCI-18-54-1211

Plot Date/Time/User: 27-Mar-12/7:39 AM/TECH2

V:\1 RBA PROJECTS\11 PROJECTS\11048 ABENGOA MOJAVE SOLAR\DRAWINGS\11048_1211_FLOOR PLAN.DWG



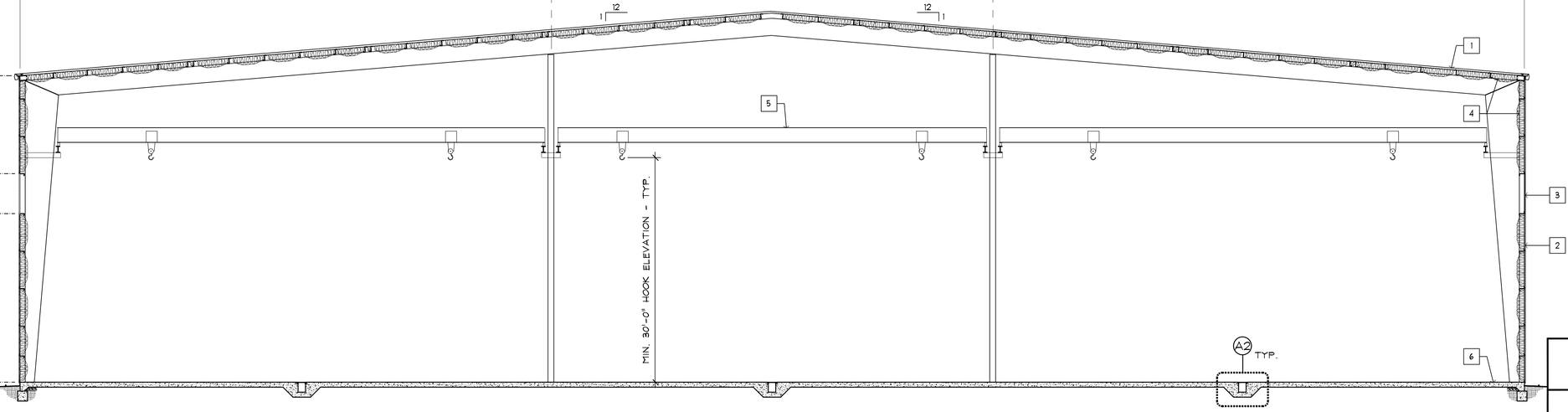
B BUILDING SECTION
SCALE: 1/8" = 1'-0"



A BUILDING SECTION
SCALE: 1/8" = 1'-0"

KEY NOTES

1. PRE-FINISHED METAL ROOF PANELS BY P.E.B.
2. PRE-FINISHED METAL WALL PANELS BY P.E.B.
3. TRANSLUCENT WALL PANEL ALL AROUND
   - BY P.E.B.
   - CONFORM TO ASTM D 3841
   - TYPE CCI, GRADE 1 W/ UV INHIBITOR, WEIGHING LESS THAN 8 OZ. / S.F.
4. R-13 WHITE PSK FACED BLANKET INSUL. @ WALLS & ROOF THRU-OUT
5. OVERHEAD BRIDGE CRANE REFER TO OWNER SPEC. 6007-ESP-ATP-18-54-1330, TYP. AS SHOWN
6. CONC. SLAB ON GRADE - SEE STRUCT. DRAWINGS & SPEC. SECTION 03300 - OWNER OPTIONAL FLOOR FINISH:
   - FLOOR HARDENER APPLICATION & DUST PROOFING
   - VAPOR RETARDANT UNDER FLOOR SLABS
   - SEALANT APPLIED @ SAWED JOINTS
3 7. WALL MOUNTED EXHAUST FAN - SEE MECH. DRAWINGS



C BUILDING SECTION
SCALE: 1/8" = 1'-0"

REVISIONS

NO.	DATE	DESCRIPTION	BY	BY	BY
1	15.2.12	1ST REVIEW & SHEET NUMBERS		PK.	15/1
3	26.3.12	OWNER REVISIONS		PK.	26/3

PE STAMP

LICENSED ARCHITECT — EDWARD J. TEYMA — No. C-27757 — EXP. 4/30/13 — STATE OF CALIFORNIA
03/26/2012

WESTAR ARCHITECTS
WESTAR Architectural Group Nevada, Inc.
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14/03/12	FOR CONSTRUCTION
ISSUE DATE	PURPOSE OF ISSUE

ABENER TEYMA MOJAVE

ABENGOA SOLAR

BUILDING SECTIONS

MOJAVE SOLAR, LLC — HINKLEY, CALIFORNIA

	BY	DATE	APP'D.	DATE	JOB NO.	REVISION
DRAWN	JC	18/01/12	JC	18/01/12	120010	3
CHECKED	RB	18/01/12	RB	18/01/12		
SCALE	AS NOTED					

DRAWING NUMBER
6007-PLN-CCI-18-54-1241

Plot Date/Time/User: 27-Mar-12/7:37 AM/TECH2
V:\1 RBA PROJECTS\11 PROJECTS\11048 ABENGOA MOJAVE SOLAR\DRAWINGS\11048_1241_BLDG. SECTIONS.DWG

LAST SAVED BY GRANTB

GENERAL FOUNDATION NOTES

1. THE METAL BUILDING SUPPLIER ASSUMES NO RESPONSIBILITY OR LIABILITY FOR FOUNDATION, FLOOR OR SLAB DESIGN OR CONSTRUCTION.
2. THE FOUNDATION DESIGN SHOULD BE DONE WITH DUE REGARD TO THE SPECIFIC SOIL CONDITIONS PRESENT AT THE ACTUAL JOBSITE.
3. FOUNDATION MUST BE DESIGNED FOR THE APPLICABLE REACTIONS AS THEY APPLY TO A PARTICULAR BUILDING AND MUST BE ADEQUATE TO RESIST ALL OF THE CRITICAL COMBINATIONS FOR EACH OF THE VARIOUS LOADING CONDITIONS. THESE REACTIONS AND LOAD COMBINATIONS MUST BE USED TO DETERMINE THE DESIGN LOADS TO BE RESISTED BY THE FOUNDATIONS.
4. REINFORCING BARS, WIRE MESH, ANCHOR ROD SHEAR ANGLES, TIE RODS AND / OR HAIRPINS (HOOK BARS) SHOULD BE INCORPORATED AS REQUIRED INTO THE FOUNDATION DESIGN. THE HORIZONTAL THRUST AT THE COLUMN BASE ACTING IN CONJUNCTION WITH APPLICABLE VERTICAL REACTIONS, MUST BE SUSTAINED BY HAIRPINS, TIE RODS, BUTTRESSES, OR OTHER DEPENDABLE MEANS.
5. COLUMN FOOTING SHOULD EXTEND A MINIMUM OF 12 INCHES INTO NATURAL SOIL, OR WHERE FILL IS USED, THE FILL MUST BE PROPERLY COMPACTED OR THE FOOTING SHALL EXTEND TO THE NATURAL GRADE. IN ALL CASES THE FOOTING SHALL EXTEND AT LEAST 6 INCHES BELOW THE LOCAL FROST LINE.
6. EXPANSION OR CONSTRUCTION JOINTS SHALL BE LOCATED AS REQUIRED IN FOUNDATION WALLS AND SLAB, AS SPECIFIED BY THE FOUNDATION DESIGNER.
7. THE TOP OF THE FOUNDATION OR FLOOR SHALL BE SQUARE, LEVEL AND SMOOTH. ANCHOR RODS SHALL BE ACCURATELY SET TO A TOLERANCE ±1/16 INCH ON DIMENSIONS WITHIN THE GROUP SPACING FOR AN INDIVIDUAL COLUMN. ALL OTHER DIMENSIONS SHALL HAVE A ±1/8 INCH TOLERANCE.
8. COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED THE ALLOWABLE BEARING STRESS OF CONCRETE THAT HAS A MINIMUM COMPRESSIVE STRENGTH OF 2500 P.S.I. AT 28 DAYS.
9. UNLESS EXPLICITLY NOTED OTHERWISE, ALL EMBEDDED STRUCTURAL STEEL (INCLUDING ANCHOR RODS), OTHER MATERIALS, AND LABOR SHALL BE SUPPLIED BY THE FOUNDATION CONTRACTOR.
10. ANCHOR RODS SHOULD BE AS SHOWN AND CALLED FOR, INCLUDING PROJECTION FROM CONCRETE, DIAMETER AND QUANTITY.
11. THE EMBEDMENT OF THE ANCHOR RODS IN THE CONCRETE AND CONFIRMING ADEQUACY OF ANCHOR ROD EDGE DISTANCE IS THE RESPONSIBILITY OF THE FOUNDATION DESIGNER. THE FRAME REACTIONS ARE CONSIDERED THE MINIMUM LOADS TO BE DEVELOPED.
12. ALL ANCHOR RODS SHALL BE ASTM F1554 GRADE 36 OR EQUAL IN ORDER TO CONFORM TO THE METAL BUILDING SUPPLIER'S DESIGN ASSUMPTIONS BASED ON THE ALLOWABLE STRESSES GIVEN IN THE AISC MANUAL OF STEEL CONSTRUCTION.
13. ANCHOR ROD DIAMETERS FOR THE PRIMARY FRAMING AND ENDWALL FRAMING ARE DENOTED AT RESPECTIVE BASE PLATE DETAILS OR ON THE ANCHOR BOLT PLAN. ANCHOR RODS FOR FRAMED OPENINGS SHALL BE 1/2 INCH DIAMETER UNLESS OTHERWISE NOTED.

BASIC MATERIAL SPECIFICATIONS

PRIMARY FRAMING STEEL

STEEL FOR MILL-ROLLED STRUCTURAL SECTIONS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A 36, ASTM A 572 GRADE 50 OR 55, OR ASTM A 992.

STEEL FOR ALL BUILT-UP SECTIONS SHALL CONFORM TO ONE OR MORE OF THE FOLLOWING:
- A. ASTM A 1011 SS, GRADE 55
- B. ASTM A 1011 HSLAS, GRADE 55, CLASS 1
- C. ASTM A 572 GRADE 55
- D. ASTM A 529 GRADE 55

STEEL FOR ENDWALL "C" SECTIONS SHALL CONFORM TO ASTM A 1011 SS, GRADE 55, OR HSLAS, GRADE 55, CLASS 1.

STEEL FOR ROUND PIPE SECTIONS SHALL CONFORM TO ASTM A 500 GRADE B, 42 KSI.

SECONDARY FRAMING STEEL

STEEL USED TO FORM PURLINS, GIRTS, EAVE STRUTS AND "C" SECTIONS SHALL CONFORM TO ASTM A1011 SS, GRADE 55, OR HSLAS GRADE 55, CLASS 1, OR IF GALVANIZED SHALL CONFORM TO ASTM A653 SS, GRADE 55, G90 OR HSLAS, GRADE 55, CLASS 1, G90.

ROOF AND WALL PANEL MATERIAL

EXTERIOR PANELS SHALL CONFORM TO ONE OF THE FOLLOWING:

PANEL MATERIAL SHALL BE ALUMINUM-ZINC ALLOY-COATED STEEL CONFORMING TO THE REQUIREMENTS OF ASTM A 792 SS, GRADE 80. MATERIAL MAY BE EITHER 26 OR 24 GAGE.

PANEL MATERIAL SHALL BE ALUMINUM-ZINC ALLOY-COATED STEEL CONFORMING TO THE REQUIREMENTS OF ASTM A 792 SS, GRADE 50, CLASS 1. MATERIAL MAY BE EITHER 24 OR 22 GAGE.

PANEL MATERIAL SHALL BE ZINC-COATED (GALVANIZED) STEEL, COATING DESIGNATION G90, CONFORMING TO THE REQUIREMENTS OF ASTM A 653 SS, GRADE 80, CLASS 1 OR CLASS 2, OR HSLAS, GRADE 80. MATERIAL MAY BE EITHER 26 OR 24 GAGE.

PANEL MATERIAL SHALL BE ZINC-COATED (GALVANIZED) STEEL, COATING DESIGNATION G90, CONFORMING TO THE REQUIREMENTS OF ASTM A 653 SS, GRADE 50, CLASS 1 OR CLASS 3, MATERIAL MAY BE EITHER 24 OR 22 GAGE.

BRACE MATERIALS:

BRACE CABLES:	ASTM A 475, 7-STRAND EHS WIRE CABLE	BC4 = 1/4" DIA.(7mm)	BC5 = 5/16" DIA.(8mm)	BC6 = 3/8" DIA.(10mm)	BC8 = 1/2" DIA.(13mm)
BRACE RODS:	ASTM A 572, 50 KSI (UPSET THREADS)	BR5 = 5/8" DIA.(16mm)	BR6 = 3/4" DIA.(20mm)	BR7 = 7/8" DIA.(23mm)	BR8 = 1" DIA.(26mm)
	OR ASTM A 529, 50 KSI (CUT THREADS)	BR9 = 1 1/8" DIA.(29mm)	BR10 = 1 1/4"DIA.(32mm)	BR11 = 1 3/8" DIA.(35mm)	BR12 = 1 1/2" DIA.(39mm)

STRUCTURAL PRIMER NOTE:

"SHOP COAT PRIMER IS INTENDED TO PROTECT THE STEEL FRAMING DURING TRANSPORTATION TO THE JOBSITE AND FOR NOT MORE THAN 45 DAYS FROM THE APPLICATION. STORAGE IN EXTREME COLD TEMPERATURES OR WINTER SNOW CONDITIONS, INCLUDING TRANSPORTATION ON SALTED OR CHEMICALLY TREATED ROADS WILL ADVERSELY AFFECT THE DURABILITY AND LONGEVITY OF THE PRIMER. THE COAT OF SHOP PRIMER DOES NOT PROVIDE THE UNIFORMITY OF APPEARANCE, OR THE DURABILITY AND CORROSION RESISTANCE OF A FIELD APPLIED FINISH COAT OF PAINT OVER A SHOP PRIMER. MINOR ABRASIONS TO THE SHOP COAT PRIMER CAUSED BY HANDLING, LOADING, SHIPPING, UNLOADING AND ERECTION ARE UNAVOIDABLE AND ARE NOT THE RESPONSIBILITY OF THE METAL BUILDING SUPPLIER. ABC IS NOT RESPONSIBLE FOR THE DETERIORATION OF THE PRIMER OR CORROSION THAT MAY RESULT FROM NEITHER ATMOSPHERIC AND ENVIRONMENTAL CONDITIONS NOR THE COMPATIBILITY OF THE PRIMER TO ANY FIELD APPLIED COATING."

"AS A MINIMUM AND SECONDARY TO MORE STRENUOUS JOB SPECIFIC REQUIREMENTS, PROJECTS LOCATED IN CANADA MUST BE ERECTED WITHIN TOLERANCES AS DEFINED IN SECTION 29.7 OF SPECIFICATION S16.01 AND PROJECTS IN OTHER LOCATIONS MUST BE ERECTED WITHIN TOLERANCES AS DEFINED IN THE LATEST EDITION OF THE METAL BUILDING SYSTEMS MANUAL, PUBLISHED BY THE MBMA."

CONSTRUCTION BRACING NOTE:

TEMPORARY SUPPORTS, SUCH AS TEMPORARY GUYS, BRACES, FALSEWORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION IS TO BE DETERMINED, FURNISHED AND INSTALLED BY THE ERECTOR. THESE SUPPORTS MUST SECURE THE STEEL FRAMING, OR ANY PARTLY ASSEMBLED STEEL FRAMING, AGAINST LOADS COMPARABLE IN INTENSITY TO THOSE FOR WHICH THE STRUCTURE WAS DESIGNED RESULTING FROM WIND AND OR SEISMIC ACTIVITY AND AGAINST THE LOADS RESULTING FROM THE ERECTION OPERATION.

CERTIFICATION AND SCHEDULE OF DRAWINGS

THIS IS TO CERTIFY THAT THE METAL BUILDING COMPONENTS FURNISHED BY AMERICAN BUILDINGS COMPANY FOR THE REFERENCED BUILDING HAVE BEEN DESIGNED IN OUR MODESTO, CA OFFICE FOR FABRICATION IN OUR CARSON CITY, NV PLANT.

IN ADDITION TO THE DEAD LOAD (D) OF THE BUILDING COMPONENTS, THE MEMBERS ARE DESIGNED ON THE FOLLOWING DESIGN BASIS:

COLLATERAL LOAD (C)
5 psf On Roof Members
5 psf On Supporting Frames
(2) 2.8 KIP USER LOAD

ROOF LIVE LOAD (Lr)
20 psf on Roof Members With Reduction On Supporting Frames As Permitted By Code

SNOW LOAD (S)
0 psf Roof Snow Load
Roof Exposure Condition = Partially Exposed
Thermal Condition = All structures that do not otherwise qualify as either "Structures kept just above freezing and others" or "Unheated structures and structures intentionally kept below freezing"
0 psf Ground Snow Load
Thermal Factor = 1.0
Snow Exposure Factor = 1.0
Snow Importance Factor = 1.0

WIND LOAD (W)
Exposure (Surface Roughness) Category = C
85 mph Basic Wind Speed (3-second gust)
Enclosure Classification = Enclosed Buildings
Internal Pressure Coefficients, GCpi = +0.18 and -0.18
Wind Importance Factor = 1.0
Design Pressure for Wall Components and Cladding Supplied by Others = +17.80 psf and -23.74 psf

SEISMIC LOAD (E)
Equivalent Lateral Force Procedure
117.4 %g 0.2s Short Period Spectral Response Acceleration S(s)
44.39 %g 1.0s Spectral Response Acceleration S(1)
Site Classification = D
Seismic Importance Factor = 1.0
Seismic Design Category = D
Seismic Design Short Period Acceleration, Sds = 0.806g
Seismic Design 1 Sec Period Acceleration, Sd1 = 0.461g
Transverse Direction OMF (Rigid Frames)
Response Modification Factor, R = 3.5; Response Coefficient, Cs = 0.223;
Longitudinal Direction OCBF (X-Bracing)
Response Modification Factor, R = 3.25; Response Coefficient, Cs = 0.248;

Classification of Building = II. All buildings and other structures except those listed in Categories I, III, and IV

NOTES:
1. ROOF DESIGN IS BASED ON THE LARGER OF LIVE LOAD OR ROOF SNOW LOAD.
2. ALL WELDING MUST BE PERFORMED BY AWS QUALIFIED WELDERS FOR THE WELDING PROCESSES AND POSITIONS TO BE USED. ALL WELDING AND WELD PREP MUST BE COMPLETED AND VISUALLY INSPECTED TO AWS ACCEPTANCE CRITERIA (TABLE 6.1) IN ACCORDANCE WITH THE APPLICABLE AWS STANDARD. WELD ELECTRODES USED FOR ALL FIELD WELD PROCESSES MUST BE SELECTED FROM TABLE 3.1 IN AWS D1.1 FOR GROUP II MATERIAL GREATER THAN OR EQUAL 0.125" THICK OR TABLE 1.2 IN AWS D1.3 FOR MATERIAL LESS THAN 0.125" THICK AND ALL FILLER MATERIAL MUST HAVE A Fu OF 70 KSI.

AMERICAN BUILDINGS COMPANY SERVICEABILITY STANDARDS (2006 MBMA MANUAL CRITERIA) WILL BE USED FOR DESIGN AND FABRICATION OF YOUR ORDER.

THE ABOVE DESIGN LOADS ARE APPLIED IN ACCORDANCE WITH THE 2010 CALIFORNIA BUILDING CODE. THE DESIGN IS IN GENERAL ACCORDANCE WITH 2005 AISC 360-05 SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS, AND 2007 AISI NASPEC NORTH AMERICAN COLD-FORMED STEEL SPECIFICATION.

THIS CERTIFICATION IS LIMITED TO THE STRUCTURAL DESIGN OF THE FRAMING AND COVERING PARTS MANUFACTURED BY AMERICAN BUILDINGS COMPANY. ACCESSORY ITEMS SUCH AS DOORS, WINDOWS, LOUVERS, TRANSLUCENT PANELS AND VENTILATORS ARE NOT INCLUDED. ALSO EXCLUDED, ARE OTHER PARTS OF THE PROJECT NOT PROVIDED BY AMERICAN BUILDINGS COMPANY SUCH AS FOUNDATIONS, MASONRY WALLS, MECHANICAL EQUIPMENT AND THE ERECTION AND INSPECTION OF THE BUILDING. THE BUILDING SHALL BE ERECTED ON A PROPERLY DESIGNED FOUNDATION IN ACCORDANCE WITH AMERICAN BUILDINGS COMPANY'S "GENERAL ERECTION GUIDE", THE LATEST EDITION OF THE MBMA MANUAL, AND THE JOB ERECTION DRAWINGS. THE DRAWINGS LISTED ON THIS SHEET SHALL REMAIN WITH AND BECOME PART OF THIS CERTIFICATION.

PRODUCT APPROVALS AND CERTIFICATIONS:
1. LONG SPAN / LONG SPAN III PANEL WIND UPLIFT-CLASS 90 (UL 90) CONSTRUCTION NO.S 71, 161, AND 167 AS LISTED IN UNDERWRITERS LABORATORIES ROOFING MATERIALS AND SYSTEM DIRECTORY.

SHEET NUMBER	DATE	REV. NO.	SHEET NUMBER	DATE	REV. NO.
5001	04/10/12	6	5031	02/16/12	3
5002	02/16/12	3	5032	02/16/12	3
5003	04/10/12	4	5033	02/16/12	3
5004	02/16/12	4	5034	02/16/12	3
5005	02/16/12	3	5035	02/16/12	3
5006	02/16/12	3	5036	02/16/12	3
5007	02/16/12	3			
5008	02/16/12	3			
5009	02/16/12	3			
5010	02/16/12	3			
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5026	02/16/12	3			
5027	02/16/12	3			
5028	02/16/12	3			
5029	02/16/12	3			
5030	02/16/12	3			

THE CERTIFYING ENGINEER HEREWITH IS NOT THE ENGINEER OF RECORD FOR THE OVERALL PROJECT AND IS ONLY CERTIFYING THAT THE DESIGN OF THE METAL BUILDING COMPONENTS FURNISHED BY AMERICAN BUILDINGS COMPANY SATISFY THE DESIGN REQUIREMENTS SPECIFIED ABOVE AND ON THE A.B.C. CONTRACT. THIS CERTIFICATION EXCLUDES THE ERECTION OF THE STRUCTURE.

REVISIONS

NO.	DATE	DESCRIPTION	BY	BY	BY
5	2/27/12	SHOW F.O. FLASHING/PANEL CUT	MDG		
4	2/16/12	RE-NUMBER PAGES	JK		
3	1/24/12	PERMIT W/ SEAL	MDG		
2	1/18/12	RE-ISSUE FOR PERMIT	MDG		
1	1/5/12	REACTION FOR USER APPLIED LOAD	TTB		

REVISIONS

NO.	DATE	DESCRIPTION	BY	BY	BY
6	4/10/12	REVISE DIMENSION AT A.B. LAYOUT	MDG		

REVISIONS

NO.	DATE	DESCRIPTION	BY	BY	BY

PE STAMP

REGISTERED PROFESSIONAL ENGINEER
RYAN P. LAWTON
C 64429
EXP 06-30-2013
CIVIL
STATE OF CALIFORNIA

AMERICAN BUILDINGS COMPANY

COLTON Constructors INC.
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Since 1970
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TEL: 480.967.8199 FAX: 480.967.8671

ABENER TEYMA
MOJAVE

02/17/12	FOR CONSTRUCTION
ISSUE DATE	PURPOSE OF ISSUE

ABENER TEYMA
MOJAVE

ABENGOA SOLAR

MOJAVE SOLAR, LLC — HINKLEY, CALIFORNIA

	BY	DATE	APP'D.	DATE	JOB NO.	REVISION
DRAWN	TTB	12/29/11			120010	6
CHECKED					516213-01	
SCALE	AS NOTED					

DRAWING NUMBER
6007-PLN-CCI-18-54-5001

Plot Date/Time/User: 18-Jan-12/2:52 PM/#####

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GENERAL STRUCTURAL NOTES

Applies unless noted otherwise on drawings

BUILDING CODE:

2010 EDITION OF THE CALIFORNIA BUILDING CODE.

LOADS:

LATERAL:
WIND:
3 SECOND WIND GUST = 85 MPH.
WIND IMPORTANCE FACTOR = 1.0.
EXPOSURE C.

SEISMIC:
SEISMIC IMPORTANCE FACTOR = 1.0.
SHORT PERIOD SPECTRAL ACCELERATION Ss = 1.17.
ONE SECOND SPECTRAL ACCELERATION S1 = 0.44.
SOIL SITE CLASS D.
Sds = 0.81.
Sd1 = 0.46.
SEISMIC DESIGN CATEGORY D.

FOR DEFLECTION/CAMBER CRITERIA OF STRUCTURAL MEMBERS ENGINEERED BY OTHERS, SEE SPECIFIC MEMBER'S SECTION BELOW.

FOUNDATIONS:

SOIL REPORT BY NINYO & MOORE ; JOB NO. 105879006. SHALLOW SPREAD OR CONTINUOUS FOOTINGS, FOUNDED IN COMPACTED FILL MAY BE DESIGNED USING AN ALLOWABLE BEARING CAPACITY OF 2,500 POUNDS PER SQUARE FOOT (PSF). THE ALLOWABLE BEARING CAPACITY MAY BE INCREASED BY 500 PSF FOR EVERY FOOT OF INCREASE IN DEPTH OR WIDTH UP TO A VALUE OF 3,500 PSF. THESE ALLOWABLE BEARING CAPACITIES MAY BE INCREASED BY ONE-THIRD WHEN CONSIDERING LOADS OF SHORT DURATION SUCH AS WIND OR SEISMIC FORCES.
SPREAD FOOTINGS SHOULD BE FOUNDED 18 INCHES BELOW THE ADJACENT GRADE AND SHOULD BE 24 INCHES OR MORE IN WIDTH. REFER TO SOILS REPORT FOR ADDITIONAL INFORMATION PRIOR TO COMMENCEMENT OF EARTHWORK. SOILS ENGINEER SHALL INSPECT FOUNDATION EXCAVATIONS PRIOR TO PLACEMENT OF CONCRETE.

CONCRETE:

SPECIFIED 28 DAY COMPRESSIVE STRENGTH F'c:

FOUNDATIONS (DESIGN BASED ON 2,500 PSI)------------------------- 4,000 PSI
SLAB ON GRADE --- 4,000 PSI

GENERAL:

USE TYPE II CEMENT AND W/C RATIO = 0.5 BY WEIGHT TYP.

ALL CAST-IN-PLACE CONCRETE CONSTRUCTION SHALL CONFORM TO THE LATEST EDITION OF THE ACI. MECHANICALLY VIBRATE ALL CONCRETE WHEN PLACED UNLESS NOTED OTHERWISE. ADMIXTURES CONTAINING CHLORIDES SHALL NOT BE USED. NO OTHER ADMIXTURES PERMITTED WITHOUT APPROVAL. FOR CONCRETE WITHOUT PLASTICIZER, MAXIMUM SLUMP 4 1/2" AT POINT OF PLACEMENT U.N.O. IF PLASTICIZER IS USED, A HIGHER FINAL SLUMP MAY BE ALLOWED UPON STRUCTURAL ENGINEER'S APPROVAL.

FOR REINFORCING INFORMATION, SEE REINFORCING SECTION OF G.S.N., PLANS, SCHEDULES AND DETAILS.

UNLESS NOTED OTHERWISE ON THE DRAWINGS, THE EMBEDMENT OF CONDUITS, PIPES, SLEEVES, ETC. OF ANY MATERIAL SHALL NOT BE PERMITTED WITHIN ANY CONCRETE STRUCTURAL ELEMENT (IE: COLUMNS, BEAMS, ELEVATED SLABS, ETC.) OR STRUCTURAL CONCRETE TOPPINGS WITHOUT THE EXPRESSED APPROVAL OF THE STRUCTURAL ENGINEER.

FLY ASH - IF PERMITTED BY ARCHITECTURAL SPECIFICATIONS, SHALL BE LIMITED TO 25% OF TOTAL CEMENTITIOUS MATERIALS BY WEIGHT. FLY ASH SHALL BE INCLUDED IN THE CALCULATION OF W/C RATIOS SPECIFIED ABOVE. FLY ASH ADDITIVES SHALL NOT BE USED ON SLABS WITH A BURNISHED OR ACID FINISH.

TEST DATA FOR EACH CONCRETE MIX SHALL BE SUBMITTED FOR REVIEW PER CHAPTER 5 OF ACI 318. REFERENCE FIGURE R5.3 FOR SUBMITTAL REQUIREMENTS AND OPTIONS. CONCRETE MIX DESIGNS THAT ARE SUBMITTED WITHOUT THE APPROPRIATE TEST DATA CANNOT BE REVIEWED.

SLABS ON GRADE:

MAXIMUM SLUMP WITHOUT PLASTICIZER AT POINT OF PLACEMENT SHALL BE 5 INCHESMIX DESIGNS SHALL TAKE CARE TO PROVIDE THE LARGEST POSSIBLE SIZE OF COURSE AGGREGATE WHILE MAINTAINING CONCRETE WORKABILITY. NOMINAL MAXIMUM AGGREGATE SIZE SHALL NOT BE LESS THAN 3/4 INCH NOR MORE THAN 1/3 THE DEPTH OF THE SLAB. MIX DESIGNERS SHALL SUBMIT SLAB ON GRADE DESIGNS WITH SHRINKAGE CHARACTERISTICS NOT EXCEEDING 0.00078 IN/IN TO MEET THE REQUIREMENTS OF ACI 360R-06, FIG5.6 FOR TYPICAL CONCRETE. SLABS SHALL BE PLACED ON A FLAT, SMOOTH, FIRM, COMPACTED SUBGRADE.

CONCRETE SHALL BE MIXED, PLACED, FINISHED AND CURED PER LATEST EDITION OF ACI 302.1 FOR THE APPROPRIATE FLOOR CLASS TYPE PER TABLE 1.1 AND SECTION 7. CURING COMPOUND SHALL BE COMPATIBLE WITH ARCHITECTURAL FLOOR FINISH.

SLABS ON GRADE SHALL BE VIBRATED ONLY AT TRENCHES, FLOOR DUCTS, TURNDOWNS, ETC. CAST CLOSURE POUR AROUND COLUMNS AFTER COLUMN DEAD LOAD IS APPLIED. UNLESS APPROVED OTHERWISE IN WRITING BY THE ARCHITECT, ALL CONCRETE SLABS ON GRADE SHALL BE BOUND BY CONTROL JOINTS (CONSTRUCTION OR SAW CUT) PER TYPICAL DETAILS, AS SHOWN ON THE FOUNDATION PLAN, SUCH THAT THE ENCLOSED AREA DOES NOT EXCEED 150 SQUARE FEET. CONSTRUCTION CONTROL JOINTS NEED ONLY OCCUR AT EXPOSED EDGES DURING POURING, ALL OTHER JOINTS MAY BE SAW CUT. SLAB REINFORCING, WHERE SHOWN, SHALL NOT EXTEND MORE THAN 125 FEET WITHOUT STOPPING THE REINFORCEMENT AT A CONTROL JOINT.

JOINTS SHALL BE FILLED OR SEALED AS SPECIFIED IN ARCHITECTURAL SPECIFICATIONS. AT A MINIMUM, JOINTS IN SLABS SUBJECT TO SOLID RUBBER, HARD URETHANE, OR NYLON CASTERS OR STEEL WHEEL TRAFFIC SHALL BE FILLED WITH A SEMI-RIGID EPOXY OR POLYUREA CONSISTING OF 100 PERCENT SOLIDS AND A MINIMUM SHORE HARDNESS OF A-80 PER ASTM D 2240. FILLER MATERIAL SHOULD BE INSTALLED THE FULL JOINT DEPTH, WITHOUT A BACKER ROD, AND FLUSH WITH THE FLOOR SURFACE VIA OVERFILLING THEN SHAVED FLAT. JOINT FILLING SHOULD BE DELAYED AS LONG AS POSSIBLE TO ACCOMMODATE THE MAXIMUM POSSIBLE SLAB SHRINKAGE.

SLAB DOWELS WHERE SPECIFIED, SHALL CONSIST OF TAPERED PLATE DOWELS AT SAWCUT JOINTS AND PLATE DIAMOND DOWELS AT FORMED CONSTRUCTION JOINT. TAPERED PLATE DOWELS SHALL BE INSTALLED WITH REMANUFACTURED DOWEL BASKETS AND DIAMOND PLATE DOWELS SHALL BE ATTACHED TO THE FORM WITH A PREFABRICATED SLEEVE. DOWELS, BASKETS, AND SLEEVES SHALL BE MANUFACTURED BY PNA CONSTRUCTION TECHNOLOGIES (PNA-INC.COM) OR APPROVED EQUAL.

VAPOR BARRIER IF REQUIRED BY ARCHITECTURAL SPECIFICATION OR SOILS REPORT SHALL CONSIST OF A MINIMUM 10 MIL MATERIAL LAPPED A MINIMUM OF 6 INCHES AND TAPED PER MANUFACTURER RECOMMENDATIONS. THE BARRIER SHALL BE PLACED ON TOP OF A SMOOTH AND COMPACTED SUBGRADE SURFACE. THE FLOOR SLAB SHALL BE PLACED OVER A TWO INCH SAND LAYER OVER A 10-MIL POLYETHYLENE VAPOR BARRIER OVER A FOUR INCH LAYER OF COMPACTED AGGREGATE BASE COURSE. ANY DAMAGE TO VAPOR BARRIER SHALL BE REPAIRED PRIOR TO AGGREGATE COURSE PLACEMENT. CARE SHALL BE TAKEN TO KEEP MOISTURE AWAY FROM THE COMPACTED SUBBASE. SUBGRADE MUST BE ALLOWED TO DRY AFTER RAINS PRIOR TO SLAB PLACEMENT. FLOOD CURING IS NOT ALLOWED. SAND IS NOT AN ALTERNATIVE FOR THE SUBBASE COURSE.

FLOOR CONCRETE SHALL USE NOVOCON XR STEEL FIBER REINFORCEMENT AS MANUFACTURED BY PROPEX CONCRETE CORP. (TEL 800-621-1273). USE 42 LB. REINFORCING STEEL PER CU. YARD OF CONCRETE.

REINFORCING:

ALL REINFORCING PER CRSI SPECIFICATIONS AND HANDBOOK. ASTM A615 (Fy = 60 KSI / GRADE 60) DEFORMED BARS FOR ALL BARS #5 AND LARGER AND FOR ALL SLABS REINFORCING. ASTM A615 (Fy = 40 KSI / GRADE 40) DEFORMED BARS FOR ALL BARS #4 AND SMALLER. WHERE SHOWN ON DRAWINGS ALL GRADE 60 REINFORCING TO BE WELDED SHALL BE ASTM A706. NO TACK WELDING OF REINFORCING BARS ALLOWED WITHOUT PRIOR REVIEW OF PROCEDURE WITH THE STRUCTURAL ENGINEER. LATEST ACI CODE AND DETAILING MANUAL APPLY. CLEAR CONCRETE COVERAGES AS FOLLOWS:

CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH -------------------- 3"
EXPOSED TO EARTH OR WEATHER
#6 OR LARGER -- 2"
#5 AND SMALLER ------------------------------------ 1 1/2"
COLUMNS (TO TIES) ------------------------------------ 1 1/2"
ALL OTHER PER LATEST EDITION OF ACI 318

ALL REINFORCING SHALL BE CHAIRED TO ENSURE PROPER CLEARANCES. SUPPORT OF FOUNDATION REINFORCING MUST PROVIDE ISOLATION FROM MOISTURE/CORROSION BY USE OF A PLASTIC OR CONCRETE CHAIR. DUCT-TAPE COVERED REINFORCING IS NOT AN ACCEPTABLE CHAIR.

ALL DIMENSIONS REFERENCED IN DRAWINGS AS "CLEAR" SHALL BE FROM FACE OF STRUCTURE TO EDGE OF REINFORCING, AND SHALL NOT BE LESS THAN STATED, NOR GREATER THAN "CLEAR" DIMENSION PLUS 3/8". ALL OTHERS SHALL BE PLUS OR MINUS 1/4" TYPICAL UNLESS NOTED OTHERWISE.

FIELD BENDING OR STRAIGHTENING OF DEFORMED BARS SHALL BE LIMITED TO #5 BARS AND SMALLER AND SHALL BE FIELD BENT OR STRAIGHTENED ONLY ONCE. ANY BEND SHALL BE LIMITED TO 90 DEGREES. IF FIELD BENDING OR STRAIGHTENING OF #6 BARS OR LARGER IS REQUIRED, OR IF A SECOND BEND IS REQUIRED FOR #5 BARS AND SMALLER, HEAT SHALL BE APPLIED FOR BENDING OR STRAIGHTENING. CONTRACTOR SHALL SUBMIT PROCEDURE FOR APPLYING HEAT TO ENGINEER FOR REVIEW AND APPROVAL PRIOR TO BENDING OR STRAIGHTENING BARS.

LAP SPLICES IN CONCRETE:

ALL SPLICE LOCATIONS SUBJECT TO APPROVAL BY THE STRUCTURAL ENGINEER. PROVIDE BENT CORNER BARS TO MATCH AND LAP WITH HORIZONTAL BARS AT ALL CORNERS AND INTERSECTIONS PER TYPICAL DETAILS. REINFORCING BAR SPACING GIVEN ARE MAXIMUM ON CENTERS. DOWEL VERTICAL REINFORCING TO FOUNDATION WITH STANDARD 90-DEGREE HOOKS UNLESS NOTED OTHERWISE. SECURELY TIE ALL BARS IN LOCATION BEFORE PLACING CONCRETE.

LAP SPLICES, UNLESS NOTED OTHERWISE, SHALL BE CLASS "B" TENSION LAP SPLICES PER LATEST EDITION OF ACI 318.

STRUCTURAL STEEL:

GENERAL:

ALL CONSTRUCTION PER LATEST AISC STEEL CONSTRUCTION MANUAL. ALL WIDE FLANGE STEEL SHALL BE ASTM A992 (Fy = 50 KSI). ALL PIPE STEEL SHALL BE ASTM A500 (Fy = 42 KSI) OR ASTM A53, TYPE E OR S, GRADE B (Fy = 35 KSI). ALL TUBE STEEL SHALL BE ASTM A500 (Fy = 46 KSI). ALL MISCELLANEOUS STEEL UNLESS NOTED OTHERWISE SHALL BE ASTM A36 (Fy = 36 KSI). THE TERMS PIPE AND ROUND HOLLOW STRUCTURAL SHAPE (HSS) ARE USED SYNONYMOUSLY THROUGHOUT THESE DOCUMENTS ALONG WITH THE TERMS TUBE STEEL AND RECTANGULAR OR SQUARE HSS.

ALL STRUCTURAL ROLLED STEEL MEMBERS WITH Fy GREATER THAN 36 KSI ARE TO BE IDENTIFIED WITH AN ASTM SPECIFICATION MARK OR TAG PER IBC SEC. 2203.1.

UNLESS NOTED OTHERWISE, ALL BOLTS SHALL BE ASTM A307. ALL BOLTS SHALL BE INSTALLED WITH STEEL WASHERS AT SHORT SLOTTED HOLES USING SNUG TIGHT INSTALLATION, UNLESS NOTED OTHERWISE.

STEEL ERECTION NOTE:

PER OSHA, STEEL MEMBERS AND DIAGONAL BRACING CANNOT BE RELEASED FROM HOISTING CABLES UNTIL ALL BOLTS OR WELDS AT MEMBER ENDS ARE COMPLETE.

HIGH STRENGTH BOLTS:

ALL HIGH STRENGTH BOLTS SHALL BE ASTM A325N AND SHALL BE INSTALLED AS BEARING TYPE CONNECTIONS WITH THREADS INCLUDED IN SHEAR PLANE. INSTALL WASHERS AND TIGHTEN "SNUG TIGHT" PER AISC SPECIFICATIONS. NO DIRECT TENSION INDICATOR TIGHTENING DEVICES OR ALTERNATE DESIGN FASTENERS ARE PERMITTED WITH "SNUG TIGHT" APPLICATIONS WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER. FOR ADDITIONAL INFORMATION, SEE ABOVE.

WELDING:

UNLESS NOTED OTHERWISE, ALL WELDS PER LATEST EDITION OF THE AWS STANDARDS. ALL WELDING SHALL BE PERFORMED BY WELDERS HOLDING VALID CERTIFICATES AND HAVING CURRENT EXPERIENCE IN THE TYPE OF WELD SHOWN ON THE DRAWINGS OR NOTES. CERTIFICATES SHALL BE THOSE ISSUED BY AN ACCEPTED TESTING AGENCY. ALL WELDING DONE BY E70 SERIES LOW HYDROGEN RODS UNLESS NOTED OTHERWISE. FOR GRADE 60 REINFORCING BARS, USE E90 SERIES. THESE DRAWINGS DO NOT DISTINGUISH BETWEEN SHOP AND FIELD WELDS; THE CONTRACTOR MAY SHOP WELD OR FIELD WELD AT THEIR DISCRETION. SHOP WELDS AND FIELD WELDS SHALL BE SHOWN ON THE SHOP DRAWINGS SUBMITTED FOR REVIEW.

HIGH STRENGTH HEADED STUDS SHALL BE AUTOMATIC WELDED CONFORMING TO ALL REQUIREMENTS OF THE LATEST EDITION OF THE "RECOMMENDED PRACTICES FOR STUD WELDING". CONFORMANCE SHALL INCLUDE, BUT NOT BE LIMITED TO, ALL QUALITY CONTROL TESTING PROVISIONS OF THE AFOREMENTIONED PUBLICATIONS

ALL FULL (COMPLETE) PENETRATION WELDS SHALL BE TESTED AND CERTIFIED BY AN INDEPENDENT TESTING LABORATORY.

PRE-ENGINEERED BUILDING DESIGN CRITERIA:

1. PRE-ENGINEERED BUILDING MANUFACTURER SHALL BE RESPONSIBLE FOR THE ENTIRE DESIGN OF THE STEEL SUPERSTRUCTURE, ROOF, DECK, FASCIAS, SUPPORT, BRACING, LATERAL ANALYSIS AND ALL RELATED WORK.

2. THE ENTIRE SUPERSTRUCTURE, INCLUDING THE ROOF DECK, SHALL BE DESIGNED IN ACCORDANCE WITH THE BUILDING CODE. WIND UPLIFT PRESSURES FOR ENCLOSED AND UNENCLOSED BUILDING AREAS SHALL BE CONSIDERED IN ACCORDANCE WITH THE SAID CODE.

3. THE PRE-ENGINEERED BUILDING SHALL BE DESIGNED TO SUPPORT SELF WEIGHT PLUS SUPERIMPOSED DEAD, LIVE, WIND OR SEISMIC LOADING, WHICHEVER COMBINATION PRODUCES THE MOST SEVERE CONDITION, IN ACCORDANCE WITH THE LATEST RECOMMENDATIONS OF THE MBMA. CONTRACTOR SHALL SUBMIT DESIGN CALCULATIONS AND SHOP DRAWINGS FOR REVIEW AND APPROVAL PRIOR TO FABRICATION. A REGISTERED ENGINEER SHALL SEAL CALCULATIONS AND SHOP DRAWINGS. SHOP DRAWINGS SHALL SHOW ALL INFORMATION INCLUDING, BUT NOT LIMITED TO, FOUNDATION REACTIONS, DIMENSIONS, MEMBER SIZES AND PROPERTIES, FRAMING PLANS, SECTIONS AND ALL PERTINENT DETAILS.

4. STEEL PURLIN TYPE AND SPACING AND STEEL DECK SELECTION SHALL BE THE OPTION OF THE PRE-ENGINEERED BUILDING MANUFACTURER WITH APPROVAL OF ARCHITECT.

5. PRE-ENGINEERED BUILDING MANUFACTURER SHALL DESIGN AND SUPPLY ALL REQUIRED SUB-FRAMING FOR OPENINGS, INCLUDING FRAMING TO SUPPORT THE WEIGHT OF MECHANICAL EQUIPMENT.

SHOP DRAWINGS:

SHOP DRAWINGS SHALL BE SUBMITTED FOR ALL STRUCTURAL ITEMS IN ADDITION TO ITEMS REQUIRED BY ARCHITECTURAL SPECIFICATIONS. CONTRACTOR SHALL PROVIDE A MINIMUM OF 2 HARD COPY SUBMITTAL SETS OF EACH ITEM TO CTS FOR REVIEW, UNLESS NOTED OTHERWISE IN ARCHITECTURAL SPECIFICATIONS. ELECTRONIC SUBMITTALS ARE NOT ACCEPTABLE.

THE CONTRACTOR SHALL REVIEW ALL SHOP DRAWINGS PRIOR TO SUBMITTAL. ITEMS NOT IN ACCORDANCE WITH CONTRACT DOCUMENTS SHALL BE FLAGGED UPON CONTRACTOR'S REVIEW.

VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS AND FIELD CONDITIONS.

MANUFACTURER OR FABRICATOR SHALL CLOUD ANY CHANGES, SUBSTITUTIONS, OR DEVIATIONS FROM CONTRACT DOCUMENTS. ANY OF THE AFOREMENTIONED WHICH ARE NOT CLOUDED OR FLAGGED BY SUBMITTING PARTIES, SHALL NOT BE CONSIDERED APPROVED AFTER ENGINEER'S REVIEW, UNLESS NOTED ACCORDINGLY.

THE ENGINEER HAS THE RIGHT TO APPROVE OR DISAPPROVE ANY CHANGES TO CONTRACT DOCUMENTS AT ANYTIME BEFORE OR AFTER SHOP DRAWING REVIEW.

THE SHOP DRAWINGS DO NOT REPLACE THE CONTRACT DOCUMENTS. ITEMS OMITTED OR SHOWN INCORRECTLY AND ARE NOT FLAGGED BY THE STRUCTURAL ENGINEER OR ARCHITECT SHALL NOT BE CONSIDERED CHANGES TO CONTRACT DOCUMENTS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE ITEMS ARE CONSTRUCTED TO CONTRACT DOCUMENTS.

THE ADEQUACY OF ENGINEERING DESIGNS AND LAYOUT PERFORMED BY OTHERS RESTS WITH THE DESIGNING OR SUBMITTING AUTHORITY.

REVIEWING IS INTENDED ONLY AS AN AID TO THE CONTRACTOR IN OBTAINING CORRECT SHOP DRAWINGS. RESPONSIBILITY FOR CORRECTNESS SHALL REST WITH THE CONTRACTOR.

DEFERRED SUBMITTALS:

SHOP DRAWING SUBMITTALS REQUIRED BY THESE GENERAL STRUCTURAL NOTES WHICH CONTAIN DESIGN CALCULATIONS SEALED BY A REGISTERED ENGINEER OTHER THAN THE ENGINEER OF RECORD, SHALL BE SUBMITTED DURING CONSTRUCTION TO THE CITY FIELD INSPECTOR FOR REVIEW. THE DOCUMENTS WILL FIRST BE REVIEWED BY THE ENGINEER OF RECORD AND DETERMINED TO BE IN GENERAL CONFORMANCE WITH THE BUILDING DESIGN. THESE DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THEIR DESIGN AND SUBMITTAL DOCUMENTS HAVE BEEN APPROVED BY THE BUILDING OFFICIAL. THE FOLLOWING ITEMS SHALL BE SUBMITTED PER THIS SECTION:

PRE-ENGINEERED BUILDINGS

GENERAL NOTES:

THE STRUCTURAL CONSTRUCTION DOCUMENTS REPRESENT THE FINISHED STRUCTURE. EXCEPT WHERE NOTED, THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO, BRACING, SHORING FOR LOADS DUE TO CONSTRUCTION EQUIPMENT, ETC. THE STRUCTURAL ENGINEER OF RECORD SHALL NOT BE RESPONSIBLE FOR THE CONTRACTOR'S MEANS, METHODS, TECHNIQUES, SEQUENCES FOR PROCEDURE OF CONSTRUCTION, OR THE SAFETY PRECAUTIONS AND THE PROGRAMS INCIDENT THERETO (NOR SHALL OBSERVATION VISITS TO THE SITE INCLUDE INSPECTION OF THESE ITEMS).

WHERE REFERENCE IS MADE TO VARIOUS TEST STANDARDS FOR MATERIALS, SUCH STANDARDS SHALL BE THE LATEST EDITION AND/OR ADDENDA. ANY ENGINEERING DESIGN, PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW, SHALL BEAR THE SEAL OF A REGISTERED ENGINEER RECOGNIZED BY THE BUILDING CODE JURISDICTION OF THIS PROJECT.

NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL STRUCTURAL NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE PROJECT, AND/OR AS PROVIDED FOR IN THE CONTRACT DOCUMENTS. WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL STRUCTURAL NOTES AND SPECIFICATIONS, THE GREATER REQUIREMENTS SHALL GOVERN.

CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFICATION OF ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO START OF CONSTRUCTION. RESOLVE ANY DISCREPANCY WITH THE ARCHITECT. ESTABLISH AND VERIFY ALL OPENINGS AND INSERTS FOR ARCHITECTURAL, CIVIL, MECHANICAL, PLUMBING AND ELECTRICAL ITEMS WITH THE APPROPRIATE TRADE DRAWINGS AND SUBCONTRACTORS PRIOR TO CONSTRUCTION.

TYPICAL DETAILS MAY NOT NECESSARILY BE CUT ON PLANS, BUT APPLY UNLESS NOTED OTHERWISE.

CONSTRUCTION MATERIALS SHALL BE SPREAD OUT IF PLACED ON FRAMED CONSTRUCTION. LOAD SHALL NOT EXCEED THE DESIGN LIVE LOAD PER SQUARE FOOT.

OPTIONS ARE FOR CONTRACTOR'S CONVENIENCE. IF AN OPTION IS CHOSEN, CONTRACTOR SHALL BE RESPONSIBLE FOR ALL NECESSARY CHANGES, APPROVALS AND THE COORDINATION OF THE WORK WITH ALL RELATED TRADES AND SUPPLIERS.

SPECIAL INSPECTION - STRUCTURAL ONLY:

SPECIAL INSPECTION IS TO BE PROVIDED FOR THE ITEMS LISTED BELOW IN ADDITION TO THE INSPECTIONS CONDUCTED BY THE BUILDING JURISDICTION. "SPECIAL STRUCTURAL INSPECTION" SHALL NOT RELIEVE THE OWNER OR THEIR AGENT FROM REQUESTING THE BUILDING JURISDICTION INSPECTIONS REQUIRED BY SECTION 109 OF THE INTERNATIONAL BUILDING CODE. SPECIAL INSPECTION IS REQUIRED PER CHAPTER 17 FOR THE FOLLOWING:

CONCRETE CONSTRUCTION:

NO SPECIAL STRUCTURAL INSPECTION IS REQUIRED FOR THE PLACEMENT OF FOUNDATION OR SLAB ON GRADE CONCRETE. INSPECTION OF REINFORCING AND ANCHOR BOLTS IS REQUIRED PER BELOW.

1. REINFORCING STEEL: INSPECTION OF IN-PLACE REINFORCING FOR CONFORMANCE PRIOR TO THE DELIVERY OF CONCRETE TO THE JOBSITE FOR THE FOLLOWING:
A. REINFORCING FOR CONCRETE FOUNDATIONS.
B. REINFORCING FOR SLABS ON GRADE.

2. ANCHOR BOLTS: INSPECTION OF BOLTS TO BE INSTALLED IN CONCRETE PRIOR TO THE PLACEMENT OF CONCRETE AROUND BOLTS.

STEEL CONSTRUCTION:

SHOP WELDING:
ALL STRUCTURAL STEEL FABRICATORS SHALL EMPLOY AN AWS CERTIFIED INDEPENDENT TESTING LAB TO PROVIDE SHOP WELD INSPECTIONS PER CODE. INSPECTION REPORTS SHALL BE SUBMITTED TO ENGINEER OF RECORD PRIOR TO STEEL INSTALLATION.
EXCEPTION: NO SHOP INSPECTION IS REQUIRED IF THE FABRICATOR IS ON THE MUNICIPALITY APPROVED STEEL FABRICATOR LIST

FIELD WELDING (IF REQUIRED):
1. WELDING:
A. PERIODIC VISUAL INSPECTION OF ALL FIELD WELDS.
B. CONTINUOUS INSPECTION OF ALL MULTIPASS FILLET WELDS OR SINGLE PASS FILLET WELDS LARGER THAN 5/16".
C. NON-DESTRUCTIVE TESTING OF ALL COMPLETE PENETRATION WELDS BY AN AWS CERTIFIED INDEPENDENT TESTING LABORATORY AT THE CONTRACTORS EXPENSE.
D. VERIFICATION OF VALID WELDER'S CERTIFICATES.

SPECIAL INSPECTION - NON STRUCTURAL:

1. SOILS (BY OTHERS) - SPECIAL GRADING, EXCAVATION, FILLING, AND IN-PLACE DENSITY INSPECTIONS TO BE PERFORMED BY THE GEOTECHNICAL ENGINEER OF RECORD.

DUTIES AND RESPONSIBILITIES OF THE SPECIAL INSPECTOR:

A. THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK ASSIGNED TO BE CERTAIN IT CONFORMS TO THE APPROVED DESIGN DRAWINGS AND SPECIFICATION.
B. THE SPECIAL INSPECTOR IS NOT AUTHORIZED TO APPROVE DEVIATIONS FROM THE DESIGN DRAWINGS OR SPECIFICATIONS, AND ALL DEVIATIONS MUST BE APPROVED BY THE STRUCTURAL ENGINEER OF RECORD PRIOR TO PROCEEDING WITH THE WORK. ALL REQUESTS FOR DEVIATIONS SHALL BE INITIATED BY THE CONTRACTOR VIA WRITTEN REQUEST FOR INFORMATION (RFI).
C. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, AND TO THE ENGINEER OR ARCHITECT OF RECORD. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION, THEN, IF UNCORRECTED, TO THE DESIGN AUTHORITY AND THE BUILDING OFFICIAL.
D. CONTRACTOR SHALL PROVIDE THE SPECIAL INSPECTOR ACCESS TO ALL ITEMS REQUIRING SPECIAL INSPECTION. ACCESS SHALL BE PROVIDED BY IN-PLACE LADDERS, SCAFFOLDS, LIFTS AND/OR OTHER EQUIPMENT OPERATED BY THE CONTRACTOR'S PERSONNEL AS REQUIRED FOR SAFE OBSERVATION. INSPECTOR IS NOT RESPONSIBLE OR AUTHORIZED TO OPERATE CONTRACTOR'S EQUIPMENT.
E. UPON COMPLETION OF THE ASSIGNED WORK THE ENGINEER OR ARCHITECT SHALL COMPLETE AND SIGN THE APPROPRIATE FORMS CERTIFYING THAT TO THE BEST OF THEIR KNOWLEDGE THE WORK IS IN CONFORMANCE WITH THE APPROVED PLANS AND SPECIFICATIONS, AND THE APPLICABLE WORKMANSHIP PROVISIONS OF THE CODE.

FOR ADDITIONAL INFORMATION SHOWN BUT NOT NOTED, SEE GENERAL STRUCTURAL NOTES ON SHEET S1.1 AND TYPICAL DETAIL SHEETS.

THESE DRAWINGS/CALCULATIONS ARE CONSIDERED PRELIMINARY -NOT FOR CONSTRUCTION OR RECORDING UNLESS THE STRUCTURAL ENGINEER OF RECORD'S SEAL IS AFFIXED WITH WRITTEN SIGNATURE.

PROJECT NUMBER	11-1200	PROJECT MANAGER	RAD
PROJECT ENGINEER	DTN	PROJECT DRAFTER	DKS

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REGISTERED PROFESSIONAL ENGINEER
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EXP. 12/31/13
3/26/12
STRUCTURAL
STATE OF CALIFORNIA

WESTAR ARCHITECTS

PROJECT NUMBER: 11104

ABENER TEYMA MOJAVE

ABENGOA SOLAR

MOJAVE SOLAR, LLC
HINKLEY, CALIFORNIA

[redacted]

DKS 18/1/12
RAD 18/1/12

GENERAL STRUCTURAL NOTES

14/3/12 FOR CONSTRUCTION

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BY DATE APP'D. DATE JOB NO. REVISION

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SCALE

ISSUE DATE PURPOSE OF ISSUE

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